1 Introduction

As annual late spring and summer temperatures affect the Greenland ice sheet, there is extensive meltwater generation and flow over its surface. Observations show that this water often collects in supraglacial lakes. A particular lake, located near the western margin of Greenland (68.72deg N, 49.50deg W), was instrumented during 2006 by Das et al. [1]; see Figs. 1 and 2 based on their work. It provided a remarkably clear record of the rapid disappearance of the lake’s water into the ice. Our previous studies [2–4] supported quantitatively their [1] suggestion that the liquid had proceeded downward along a major crevasse system extending below the lake, through a process suggested by Weertman [5], and then propagated as a turbulently driven hydraulic fracture along the ice/rock interface at the base (see below).

The basic facts about the supraglacial meltwater lake are as follows [1] (Figs. 1 and 2):

1. The lake began filling in July 2006.
2. The lake surface level reached a maximum at ~0:00 hr on July 29, 2006.
3. At that maximum level, the lake volume was ~44 × 10⁶ m³, and surface area was ~5.6 km².
4. Shortly after ~0:00, the level (marked by the “falling lake level” curve in Fig. 2 and read on the right-side scale) was observed to slowly/steadily fall at ~15 mm/hr.
5. The rate of fall became much more rapid shortly after 16:00 hr.
6. Then, within ~1.5 hr, the lake water rapidly disappeared into the ice, with the lake surface falling at a maximum rate of ~12 m/hr, with the maximum volumetric discharge rate \( Q > 10,000 \text{ m}^3/\text{s} \), and average rate \( Q \sim 8700 \text{ m}^3/\text{s} \) during the discharge. (In comparison, for the Niagara River leading to Niagara Falls, the average \( Q \sim 5750 \text{ m}^3/\text{s} \) [6].)

The lake level \( H_{\text{Lake}} \) during drainage was determined from two pressure meters (Hobo 1 and 2), see Fig. 1, although these were left dry (Fig. 2, curve denoted \( H_{\text{Lake}} \), with axis scale on right side) well before full drainage. A global positioning system (GPS) instrument was placed near the lake edge, but yet further from it ~2.7 km long crevasse system (Fig. 1) through which the water is presumed to have drained. The uplift it recorded is labeled \( Z_{\text{rel}} \), the black curve with axis scale on the left, and its time rate is shown. We have marked the ~15 m maximum transient uplift \( Z_{\text{rel}} \) attained at ~17:40 hr, whereas the uplift rate \( dz_{\text{rel}}/dt \) is maximum at ~17:00 hr.

A simple calculation suggests that the water entering beneath the ice sheet, transiently lifting it from its bed, does so by a strongly turbulent flow. Let \( R \) be the radius of the subglacial fracture, approximated for simplicity as circular, near the condition of full lake discharge. Then, \( \pi R^2 \times \text{ uplift of 1.5m} \approx \text{ lake volume of } 44 \times 10^6 \text{ m}^3 \), giving \( R \approx 3.5 \text{ km} \) at full drainage. Since it takes about 1.2 hr to occur, the average fracture growth speed along the interface can be approximated as \( R/1.2 \text{ hr} \approx 3 \text{ km/hr} \). Recalling that the kinematic viscosity of water is \( \sim 10^{-6} \text{ m}^2/\text{s} \), the Reynolds number for flow in the fracture is

\[
\text{Re} = (3 \text{ km/hr} \times 1.15 \text{ m})/(10^{-6} \text{ m}^2/\text{s}) \approx 8 \times 10^8
\]

numbers in excess of \( 10^7 \) would still be appropriate if we reduced the assumed gap size significantly, as may be appropriate for the earlier phases of the fracture propagation.

However, although our initial attempt [2] to explain the remarkably short time scale for the lake disappearance yielded order-of-magnitude agreement, it did not agree precisely with observations. Ice, like all solids, responds elastically on short time scales, although creep deformation becomes dominant on longer time scales [7]. The assumption of elastic response seems appropriate over the short time scale of the lake drainage, of order 1.5 hr in Fig. 1. Nevertheless, we argue here that a critical aspect of the drainage process was developing by creep flow, well before the onset of rapid drainage. The creep, a process to which Needleman and coworkers \([8,9]\) have contributed...
insightful computational methodology in other contexts, plays an important role in determining the estimated time scale of the lake drainage.

2 Analysis

To model the drainage event, or at least a simple but tractable representation of it, we adopt the solution of Ref. [3] for an ice sheet of uniform thickness $H$ ($\approx$ 1 km), see Fig. 3, in which a vertical crevasse connected to the lake supplies water to a growing opening gap (basal fracture) along the bed, with fluid inlet pressure $p_{\text{inlet}}(t)$ at the entry point. The equation solved in Ref. [3] generalized that in Ref. [2] to allow the crack half-length $L(t)$ to be comparable to and several times larger than ice thickness $H$ (whereas Ref. [2] presented a self-similar solution for the range $L(t)/H \ll 1$, i.e., effectively for a fracture at the base of an unbounded domain).

The modeling of Ref. [3] assumes, as justified in retrospect, that for purposes of calculating the elastic deformation and hence the crack opening displacement, that only the local fluid pressure $p(x, t) = -\sigma_{zz}(x, z = 0, t)$ need be considered. That is, because it is normally far greater than the shear tractions $\tau_{\text{wall}}(x, t) = -\sigma_{xz}(x, z = 0, t)$ exerted along the walls of the fracture from resistance to fluid infiltration. To solve for the crack opening in a manner which solves the elasticity equations in 2D plane strain and meets the traction-free surface boundary conditions at $z = H$, the numerical integral equation formulation of Erdogan et al. [10] (with correction of a misprinted kernel as noted in Ref. [11]) was used, which relates the pressure distribution $p(x, t)$ to the crack opening gap $h(x, t)$ at each time $t$ (with inertia neglected because of the slowness of fracture propagation speeds relative to elastic wave speeds). Also, in view of the low fracture toughness of ice, $K_{\text{IC}} \approx 0.1$ MPa, it was judged that toughness became unimportant, in the sense quantified by Gagarash and Detournay [12], once crack half-length $L(t)$ was greater than $\approx 10$ m. Effectively, over the long length scale of fracture growth ($L > 1$ km), the problem of fracture becomes asymptotically indistinguishable from the problem of lift-off along a non-adhering (zero $K_{\text{IC}}$) interface.

Elasticity theory under plane strain conditions in the $x-z$ plane, and within the usual approximations of linear elastic theory, relates displacement discontinuities $\Delta u_x(x, t)$ and $\Delta u_z(x, t)$ (along the fracture plane $z = 0$, between $x = -L(t)$ and $x = +L(t)$), to the traction stress components acting on the plane and in adjacent material by

$$\begin{align*}
\sigma_{zz}(x, z, t)/E & = \left\{ \begin{array}{ll}
\sigma_{zz}(x, z, t)/E' & \\
\sigma_{zz}(x, z, t)/E'' & \end{array} \right.

\begin{aligned}
& = \int_{-L(t)}^{L(t)} \left( K_{xx}(x - x', z) K_{zz}(x - x', z) \right) \frac{\partial}{\partial x'} \left( \Delta u_x(x', t) \right) dx'. 
\end{aligned}
\end{align*}
$$

The notation here is that with $u_x = u_x(x, z, t)$, for $x = x$ or $z$, $\Delta u_x(x, t) = u_0(x, z = 0^+, t) - u_0(x, z = 0^-, t)$, the kernels $K_{xx}$, $K_{xx}$, $K_{zz}$, and $K_{zz}$ all vanish on the plane $z = H$, the surface of the ice sheet, so as to meet the traction-free boundary condition. Also, on the plane $z = 0$, the diagonal kernels $K_{xx}$ and $K_{zz}$ include terms which are Cauchy singular, like $1/(x - x')$. Here, $E = E/(1 - \nu^2)$, where $E$ is Young’s modulus and $\nu$ is the Poisson ratio.

Fig. 3 Schematic of subglacial drainage system showing the vertical influx $Q_{\text{vert}}(t)$ from the lake through the crack–crevasse system feeder channel, and the resulting water injection along the ice–bed interface. The feeder channel horizontal opening $\Delta u$ includes contributions from elastic opening, $\Delta u_{\text{el}}$, linearly proportional to the current fluid pressure, and prior creep opening, $\Delta u_{\text{cr}}$, which accumulated over an extended time before the rapid drainage. The ice sheet height $H$ is much larger than the lake depth and the basal fracture opening $h(x, t)$. Additionally, the lake diameter is also significantly smaller than the ultimate horizontal spread of $2L(t)$ of the basal hydraulic fracture.
As \( z \to 0^+ \), \( \sigma_z(x, z, t) \to -p \), where \( p \) is the local fluid pressure in the fracture, whereas \( \sigma_z(x, 0, t) \to -\tau_{\text{wall}} \), the shear stress resisting the turbulent fluid flow in the fracture. Typically, in such hydraulic fracture situations, \( \tau_{\text{wall}} \ll p \) and we follow Ref. [3] in neglecting \( \tau_{\text{wall}} \) in comparison to \( p \). Thus, recognizing \( \Delta u(x, t) \) as \( h(x, t) \), the opening gap along the fracture, the hydraulic fracture problem is formulated, like in Ref. [3], as

\[
\begin{cases}
0 & t\leq L(t) \\
-p(x, t) &= \int_{-L(t)}^{+L(t)} \left( K_x(x - x', 0^+) + K_x(x - x', 0^+) \right) \frac{\partial}{\partial x'} \left( \Delta u(x, t) \right) dx' \\
-h(x, t) &= \int_{-L(t)}^{+L(t)} \left( K_x(x - x', 0^+) + K_x(x - x', 0^+) \right) dx'
\end{cases}
\]

(3)

which ultimately relates the pressure distribution \( p(x, t) \) within the fracture to its opening gap \( h(x, t) \) along it.

Consistent with the high Reynolds number estimated in Eq. (1), the flow in the fracture is expected to be a turbulent flow in a rough-walled gap. The relevant considerations are, first, that the Darcy–Weisbach friction factor \( f \), for such flows at mean velocity \( U \) in rough-walled pipes or channels, is defined by writing the wall shear stress resisting the flow as \( \tau_{\text{wall}}/*\rho U^2/2 \) = \( f/4 \) (\( \rho \) is the density of the fluid, water in our case). The \( f \) may be estimated for the present case of thin flow in a slit by using well-calibrated data for flow in rough-walled cylindrical pipes, reinterpreted for a slit using the hydraulic radius concept. Following Ref. [2] that gives \( f \approx 0.143(k/h)^{1/3} \), where \( k \) is the amplitude of the wall roughness as an equivalent Nikuradse grain size. An insightful discussion on such turbulent flow in rough-walled tubes is given by Gioia and Chakraborty [13]; see also Ref. [14].

Observing that the gap \( h \) times the pressure gradient \( -\partial p/\partial x \) is equilibrated by \( 2\tau_{\text{wall}} \), we have

\[
-h \frac{\partial p}{\partial x} = 0.0357\rho U^2(k/h)^{1/3}
\]

(4)

(here, \( \rho \) is the mass density of the water; we use \( \rho_{\text{ice}} \) below for the lesser mass density of the ice).

To the preceding Eqs. (3) and (4), we add the conservation of mass

\[
\frac{\partial (hU)}{\partial x} + \frac{\partial h}{\partial t} = 0
\]

(5)

to close the system, as the set of equations in fluid pressure \( p \), fracture opening gap \( h \), and fluid velocity \( U \) that was formulated and solved numerically in Ref. [3] (and earlier for the \( L \ll H \) range in Ref. [2]).

3 Application to Basal Fracture Propagation

Here, we present the solutions to the above system of equations, as devised in Ref. [3], and use them subsequently to address the time scale of glacial underflooding in the lake drainage event considered.

The rate of fracture propagation along the bed is

\[
\frac{dL}{dt} \equiv U_{\text{wp}} = \left( \frac{\rho L}{\rho_{\text{inlet}} - \sigma_0} \right)^{1/3} \left( \frac{\rho_{\text{inlet}} - \sigma_o}{E} \right)^{1/3} \left( \frac{L}{k} \right)^{1/8} \eta \left( \frac{L}{H} \right)
\]

(6)

Here, the notation is a reminder that the fracture growth rate is assumed equal to the fluid velocity at the tip, and the function \( \eta(L/H) \), as fitted to the numerical results of Ref. [3], is

\[
\eta(L/H) \approx 5.13[1 + 0.125(L/H) + 0.183(L/H)^2]
\]

(7)

The polynomial in \( L/H \) in the brackets closely fits results of Ref. [3] out to \( L/H = 5 \), that is, to \( L = 5 \ km \). Here, \( \sigma_0 \) is given by

\[
\sigma_0 \equiv \rho_{\text{inlet}} g H
\]

(8)

the overburden pressure of the ice (which \( \rho_{\text{inlet}} \) must evidently exceed in order for water to be driven beneath the ice sheet to open the fracture). Note that in the absence of vertical flow, \( \rho_{\text{inlet}} = \rho g H \), and since the liquid water density \( \rho \approx 1 \rho_{\text{ice}} \), \( \rho_{\text{inlet}} > \sigma_0 \) under those hydrostatic conditions, which is what drives the water to the bed.

Further, for a given inlet pressure \( \rho_{\text{inlet}} \), the average \( h_{\text{avg}} \) of the opening gap \( h(x, t) \) along \( -L(t) < x < +L(t) \), i.e., along the fracture, is expressible in terms of \( \rho_{\text{inlet}} \) and \( L \), as

\[
h_{\text{avg}} \approx 1.73\frac{\rho_{\text{inlet}} - \sigma_0}{E}\left[1 + 0.517(L/H)^2\right]
\]

(9)

Here, the expression in the brackets closely fits numerical results of Ref. [3] up to \( L/H = 3.5 \), but falls about 15% too low at \( L/H = 5 \).

To estimate the volumetric inflow rate \( Q_{\text{basal}} \) (units \([L^3]/[T]\)) to the bed, we choose some representative width \( W \) perpendicular to the diagram of Fig. 3 (i.e., in the unmarked \( y \) direction) over which the inflow rate (units \([L^3]/[T]\)) per unit distance perpendicular to the plane of the diagram, calculated from the 2D plane strain solution of Ref. [3], may be assumed to apply approximately. We take \( W = 3 \ km \) for the width, noting that the major crevasse marked in Fig. 1 extends over a length of 2.7 km along the lake bed, and anticipating that the flow extended the lift of the ice off from its bed somewhat beyond the end of that feature. (Ultimately, a 3D analysis is needed, but that is well beyond the scope of this paper.) Thus, noting that \( 2LW h_{\text{avg}} \) is the volume of water in the subglacial fracture

\[
Q_{\text{basal}} = \frac{d(2LW h_{\text{avg}})}{dt} = 6.88\frac{\rho_{\text{inlet}} - \sigma_0}{E}\frac{W L}{1 + 0.303 L^2 / H^3} \frac{dL}{dt}
\]

(10)

d\( dL/dt \) is given by Eq. (6), with Eq. (7), above. We note that for a given \( L/H \), \( dL/dt \propto (\rho_{\text{inlet}} - \sigma_0)^{1/6} \), and thus,

\[
Q_{\text{basal}} \propto (\rho_{\text{inlet}} - \sigma_0)^{13/6}
\]

(11)

4 Coupling to Lake Water Supply to the Bed by a Vertical Crack–Crevasses System

By mass conservation, the volumetric flow rate \( Q_{\text{basal}} \) into the basal fracture must equal \( Q_{\text{vert}} \) (see Fig. 3), the volumetric flow rate at which lake water flows down the vertical crack–crevasse system (which forms a feeder channel to the basal fracture). We assume that this vertical system has a width in the \( y \) direction (perpendicular to the plane of the diagram in Fig. 3), which is the same width \( W \) as adopted above for the basal fracture. Also, for simplicity in making elementary estimates of the lake drainage time scale, we model this vertical feeder channel as having a spatially uniform opening gap \( \Delta u = \Delta u(t) \), thought of as the area-averaged opening of a vertical crack of depth \( H \approx 1 \ km \) in the \( z \) direction and width \( W \approx 3 \ km \) in the \( y \) direction, with faces loaded by the area-average pressures.

We determine the vertical flux using an elementary balance of forces on a vertical slab of water from the lake, of area \( HW \) and thickness \( \Delta u \), moving downward with velocity \( U_{\text{vert}} (= Q_{\text{vert}}/W\Delta u) \). The flow is driven downward by the slab weight \( \rho_{\text{water}}W\Delta u \), which is balanced by the sum of the upward force \( \rho_{\text{water}}W\Delta u \) at the base of the slab and the shear forces from the wall shear stresses on the two vertical boundaries summing to \( 2\tau_{\text{wall}}WH \), where \( \tau_{\text{wall}} \approx \rho f(8)\eta_{\text{vert}}U_{\text{vert}} \) and \( f = 0.143(k/H)^{1/3} \). Balancing these forces leads to

\[
\rho_{\text{water}}W\Delta u = (f/4)\rho_{\text{vert}}U_{\text{vert}}^2 WH + p_{\text{inlet}}W\Delta u
\]

(12)
which can be rearranged to give the formula for the vertical flux

\[ Q_{\text{vert}} \approx 5.29 \left(1 - \frac{p_{\text{inlet}}}{\rho g H}\right)^{1/2} W \Delta \bar{\kappa}^{3/2} \bar{u}^{1/2} \left(\frac{\Delta \bar{u}}{k}\right)^{1/6} \]  

(13)

In a somewhat similar attempt to link flow down the vertical crevasse to flow into the basal fracture, Tsai and Rice [2] assumed, in view of the relatively short time scale (<1.5 hr) of rapid drainage, purely elastic response of the vertical crevasse with its opening being proportional to the difference between the average pressure \( p_{\text{inlet}}/2 \) in the crevasse and the corresponding average \( \sigma_n/2 \) for the far-field horizontal stress in the ice, which led to the unphysical result of eventual complete crevasse closure when assuming purely elastic response of the ice (which is discussed below).

To more accurately model the drainage, particularly regarding the previous assumption of purely elastic ice deformation, we allow here for the possibility of significant creep opening of the crevasse. To motivate this, recall the slow falling of the lake level allowing here for the possibility of significant creep opening of the crevasse before nucleation of the basal fracture. (That nucleation was expected to be close to 1.0 (and is 1.0 in the linear viscous case), we use the following approach. Based on solutions for pressurized circular holes, elliptical holes, and flat cracks in the \( n = 1 \) linear viscous case (analogous to familiar linear elastic solutions, but evaluated with \( \nu = 1/2 \)), and the Nye [15] solution for a pressurized circular hole in a power law creeping material, Fernandes and Rice (in progress) have conjectured that the average crease opening rate of a pressurized crack of length \( W \) in plane strain, opened by uniform pressure taken as \( p_{\text{inlet}}/2 \), in a power law creeping solid under far-field compressive stress \( \sigma_n/2 \)

\[ \Delta \bar{\kappa}^{3/2} = \frac{\pi(p_{\text{inlet}} - \sigma_n)}{4E} W \]  

(15)

We define \( C \) as the ratio of the prior creep opening \( \Delta \bar{\kappa}^{3/2} \) over the 16 hr of slow drainage to the elastic opening \( \Delta \bar{\kappa}^{3/2} \). In that comparison, both are evaluated for hydrostatic pressure \( p_{\text{inlet}} = \rho g H \) (see Eq. (17) to follow). Inserting this formula for the average conduit opening into our formula for the vertical flux given in Eq. (13), we can write \( Q_{\text{vert}} \) as a function of \( p_{\text{inlet}} \) alone.

In numerical simulations, we compared results from the above stress-strain elastic solution for a crack of length \( W \), in a medium under far-field compressive stress \( \sigma_n/2 \), with its opening being proportional to the difference between the

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To estimate the prior creep opening, we represent the creep deformation of the ice sheet by the Glen law form typically adopted in glaciology [7]. This is \( d^1/dt = 2A_c(T) \tau^n \), with \( n = 3 \), where \( \tau \) is the Mises equivalent shear stress, based on the second invariant of the deviatoric stress tensor \( S_{ij} \), and \( d^1/dt = d\bar{\kappa}/dt \) is the equivalent engineering shear strain rate, with the trace of the creep strain rate vanishing (no volumetric creep strain) and with components of deviatoric strain rate being in proportion to one another just as are components of \( S_{ij} \). Recommended values of \( A_c(T) \) are given by Cuffey and Paterson [7], and within the simplicity of our modeling, we evaluate \( A_c(T) \) based on a single value of \( T \), hoped to be representative of ice temperatures in the vicinity of the lake and crevasse system. We note that \( A_c(T) \) remains finite for the solid phase of ice at its melting temperature.

Although there is no general analytical solution for crack opening in a material undergoing power law creep (except in the linear viscous case), we use the following approach. Based on solutions for pressurized circular holes, elliptical holes, and flat cracks in the \( n = 1 \) linear viscous case (analogous to familiar linear elastic solutions, but evaluated with \( \nu = 1/2 \)), and the Nye [15] solution for a pressurized circular hole in a power law creeping material, Fernandes and Rice (in progress) have conjectured that the average crease opening rate of a pressurized crack of length \( W \) in plane strain, opened by uniform pressure taken as \( p_{\text{inlet}}/2 \), in a power law creeping solid under far-field compressive stress \( \sigma_n/2 \), could be represented as

\[ d\bar{\kappa}^{3/2}/dt = k(n) \frac{\pi}{2} A_c(T) \left(p_{\text{inlet}} - \sigma_n\right) W \]  

(16)

where \( k(n) \) is a correction factor depending on \( n \), which is expected to be close to 1.0 (and is 1.0 in the \( n = 1 \) case).

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(16)

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In numerical simulations, we compared results from the above approximation to a plane strain finite element model of a pressurized crack using ABABUS, which was formulated as a Maxwell model (elastic and viscous elements in series, where the viscous

### Table 1

A table summarizing the parameter values used in this manuscript. All parameters choices follow those made in Ref. [2] except for the creep parameters which follow Ref. [7].

<table>
<thead>
<tr>
<th>Parameter</th>
<th>( H )</th>
<th>( W )</th>
<th>( \rho )</th>
<th>( p_{\text{ice}} )</th>
<th>( g )</th>
<th>( k )</th>
<th>( E )</th>
<th>( n )</th>
<th>( t )</th>
<th>( A_c(-7 ^\circ C) )</th>
<th>( A_c(-5 ^\circ C) )</th>
</tr>
</thead>
<tbody>
<tr>
<td>Value</td>
<td>1</td>
<td>3</td>
<td>1000</td>
<td>9.0</td>
<td>9.81</td>
<td>0.01</td>
<td>6.8</td>
<td>3</td>
<td>16</td>
<td>( 3.62 \times 10^{-25} )</td>
<td>( 9.31 \times 10^{-25} )</td>
</tr>
<tr>
<td>Unit</td>
<td>km</td>
<td>km</td>
<td>kg/m³</td>
<td>kg/m³</td>
<td>m/s²</td>
<td>m</td>
<td>GPa</td>
<td>hr</td>
<td>Pa·s⁻¹</td>
<td>Pa·s⁻¹</td>
<td></td>
</tr>
</tbody>
</table>

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elements satisfy power law creep), so that when solved within small geometry change assumptions, the displacement rates converge in time to those of a purely viscous material. This approach allows the model to respond to suddenly applied and sustained boundary loading but on a time scale before the crack has opened enough to respond differently from a straight cut. In order to isolate the creep stress rate using the ABAQUS elastic plus creep deformation formulation, we waited long enough for essentially steady state creep strain rate to be achieved and elastic relaxation to be completed. See Refs. [8] and [9] for other computational approaches to creep flow. The simulation was benchmarked using two simple tests: (1) comparing the opening rate of the crack under linear viscous deformation (\(n = 1\)) to the known analytical solution. Results yield an average nominal nodal error of 0.03% with a maximum nodal error of 0.1% nearest the crack tip. (2) Using a model, assuming a nonlinear rheology (\(n = 3\)) generates results which were found to compare favorably to the HRR field described by Hutchinson [16] and Rice and Rosengren [17]. By comparing the approximation in Eq. (16) to the numerical solution attained from the ABAQUS model, the correction factor \(k(n)\) was found, for the \(n = 3\) case of interest in this study, to be \(k(3) \approx 0.8\.

We estimate \(\Delta \dot{W}\) at the time of basal fracture nucleation, after approximately 16 hr of slow leakage from the lake, by assuming hydrostatic pressurization of the vertical crevasse system (\(p_{\text{inlet}} = \rho g H\)) for the 16 hr period, hence multiplying \(d\Delta \dot{W}/dt\) above by 16 hr. As noted, a scale for the resulting \(\Delta \dot{W}\) is to compare it to the \(\Delta \dot{W}\) corresponding to hydrostatic pressurization of the vertical crevasse, giving C as

\[
C = \frac{\Delta \dot{W}[\text{creep}]}{\Delta \dot{W}[\text{inlet}]} \quad \text{(17)}
\]

Note that \(C\) scales linearly with time \(t\) of hydrostatic pressure, with \(A_c(T)\), and with \(H^2\) (when \(n = 3\)). Also, \(A_c(T)\) has a weak dependence on ice pressure \(P\) as we show in Sec. 5, which we choose as \(P = \sigma_0/2\). Assuming \(H = 1\) km, \(E^c \approx 6.8\) GPa, \(n = 3\), and \(k(3) \approx 0.8\), we consider (being mindful that, as we show in Sec. 5, the best-fitting \(C\) to match the discharge data is in the range of \(C = 1.5-2.0\)) average ice temperatures in the range of \(-7.0^\circ\text{C}\) to \(-5.0^\circ\text{C}\). The corresponding \(A_c\) are \(A_c(-7^\circ\text{C}) = 6.32 \times 10^{-25}\) s\(^{-1}\) Pa\(^{-3}\) and \(A_c(-5^\circ\text{C}) = 9.31 \times 10^{-23}\) s\(^{-1}\) Pa\(^{-3}\). With those parameters, and with \(H = 1\) km, and standard values of \(\rho\) and \(\rho_{\text{water}}\) we obtain \(C = 1.42\) at \(-7.0^\circ\text{C}\) and \(C = 2.11\) at \(-5.0^\circ\text{C}\), which closely bracket the preferred range of \(C\). Similarly, we obtain temperatures corresponding to relevant values of \(C = 1.50\) at \(T = -6.75^\circ\text{C}\) and \(C = 2.00\) at \(T = -5.27^\circ\text{C}\) (see Table 1).

In the calculations of Sec. 5, we recognize that the creep opening does not change appreciably during the short time scale of lake drainage, and therefore represent the total crevasse opening \(\Delta \dot{W}\) (as needed in Eq. (12) above) to characterize resistance to flow down the crevasse as

\[
\Delta \dot{W} = \frac{\pi(q_{\text{inlet}} - \sigma_o)}{4E^c} + \frac{\pi(qgH - \sigma_o)}{4E^c} \quad \text{(18)}
\]

where \(\sigma_o = \rho_{\text{water}} g H\). A recent study addresses in another context how a pre-existing subglacial drainage system interacts with fluid penetration along it [18].

### 5 Estimating the Time Scale for Lake Drainage

We estimate the time scale for lake drainage using our model for combined flow in the vertical conduit and basal fracture. Equation (6) is solved for the evolution of the basal fracture using an inlet pressure \(p_{\text{inlet}}\) found by coupling the vertical conduit with the basal fracture through

\[
Q_{\text{basal}} = Q_{\text{vert}} \quad \text{(19)}
\]

Using Eqs. (10) and (13), this can be rearranged to find an equation for the inlet pressure

\[
\frac{1}{4E^c} \left(\frac{\pi W}{4H}\right)^2 ((\rho - \rho_{\text{water}})gH - \Delta \rho)^2 (\Delta \rho + C (\rho - \rho_{\text{water}})gH)^{\frac{3}{2}} = \Delta \rho \frac{2}{L/H} \quad \text{(20)}
\]

where we have defined the excess inlet pressure to be \(\Delta \rho = p_{\text{inlet}} - \rho_{\text{water}} g H\) and the function

\[
F(x) = x^2 (1 + 0.125x + 1.218x^2 + 0.129x^3 + 0.189x^4) \quad \text{(21)}
\]

For \(C = 0\), Eq. (20) can be solved analytically, allowing \(p_{\text{inlet}}\) to be written as a function of \(L\). This turns Eq. (6) into a single ordinary differential equation (ODE) for the length of the basal fracture \(L\) that is solved using built-in \textsc{matlab} routines. The solution is slightly more complicated when \(C \neq 0\), and Eqs. (6) and (20) must be solved simultaneously. It can be shown that Eqs. (6) and (20) form a system of differential-algebraic equations of index 1, leading to two possible solution methods. The first method involves treating Eq. (6) as a single ODE while solving the algebraic equation at each time-step using standard root finding methods, and the second method involves differentiating Eq. (20) with respect to time to yield an ODE for \(p_{\text{inlet}}\) that is solved alongside Eq. (6). Both methods were tested and found to give consistent results, though all results shown from this point onward were produced using the root finding method.

Figure 5 shows the evolution of \(L\) and \(p_{\text{inlet}}\) for the parameters given in Table 1 and a range of values of \(C\) between zero and two. As shown later, for the largest values of \(C\), the lake can completely drain. When total drainage occurs, we terminate the solutions and indicate this by a solid circle in Fig. 5. We observe that the basal fracture grows to a length of several kilometers within a few hours, with larger values of \(C\) leading to faster fracture growth. This is to be expected since a larger value of \(C\) corresponds to a wider vertical conduit, and thus a larger water flux delivered to the basal fracture. Interestingly, we observe that the inlet pressure is relatively insensitive to changes in \(C\), with the inlet pressure typically close to the ice overburden \(\sigma_o\). However, the sensitive dependence of \(U_{\text{tip}}\) on the difference between \(p_{\text{inlet}}\) and \(\sigma_o\) turns this small difference in excess pressure into a pronounced difference in basal fracture length. The small excess pressures shown in Fig. 5 mean that elastic opening of the vertical conduit is typically small when compared to the opening due to creep in the period immediately before rapid drainage commences. This is shown in Fig. 5 where we observe that the average conduit opening quickly returns to \(\Delta \dot{W} = \Delta \dot{W}^{\text{inlet}}\) as the inlet pressure falls toward \(\sigma_o\). We do not show the evolution of conduit opening for \(C = 0.5\) and \(C = 1.5\) since it is qualitatively similar to the solutions for \(C = 0\), 1, and 2, with the average conduit opening falling to \(\Delta \dot{W}^{\text{inlet}}\) over a time scale of approximately half an hour.

Using our solution for \(L\) and \(p_{\text{inlet}}\), we can calculate the water flux from the lake and into the basal fracture using Eq. (13). The evolution of this flux is also shown in Fig. 5. We observe that at the onset of rapid drainage, the flux increases rapidly before reaching a state where the flux remains almost constant. Our results show a strong dependence of the water flux on \(C\), which is to be expected when elastic opening of the vertical conduit is small. Importantly, in the solution for \(C = 0\), when creep is neglected and all opening of the conduit is elastic, we see that the conduit quickly closes as the inlet pressure drops preventing the lake from draining. In Fig. 5, we indicate the average water flux of \(\approx 8700\) m\(^3\)/s inferred in Ref. [1] using a dashed line and find that this is best matched by the solution with \(C = 1.5\). Our calculations for the water flux into the basal fracture can be used to predict how the lake surface height drops as the rapid drainage begins. To do this, we must first make some assumptions about the geometry of the lake. We assume an axisymmetric lake with a parabolic shape, which allows us to relate the radius of the lake \(r\) at a given distance below the lake surface \(z\), where \(z\) is taken to be positive in the upward direction and zero at the initial lake surface. For this parabolic shape
\[ r^2 = \hat{a}(z + D), \quad \hat{a} = \frac{A_{\text{init}}}{nD} \]  

where \( D \) is the initial lake depth, and \( \hat{a} \) is a constant determined using observations of the initial lake surface area \( A_{\text{init}} \). This allows us to find the area of the lake at any depth \( z \) and initial lake volume

\[ A(z) = \frac{A_{\text{init}}(z + D)}{D}, \quad V_{\text{init}} = \frac{DA_{\text{init}}}{2} \]

Thus, using the measurements of \( A_{\text{init}} = 5.6 \text{ km}^2 \) and \( V_{\text{init}} = 44 \times 10^9 \text{ m}^3 \) from Ref. [1], we estimate the initial lake depth to be \( \sim 15.7 \text{ m} \).

Having approximated the lake geometry and estimated the initial depth, we now model the lake drainage using our solution for \( Q_{\text{vert}} \) and rewrite this in terms of the lake surface height using our solution for \( A(z) \) to find

\[ \frac{dV}{dt} = -Q_{\text{vert}}(t), \quad \frac{dz}{dt} = -\frac{DQ_{\text{vert}}(t)}{A_{\text{init}}(z + D)} \]

where \( V \) is the current volume of water in the lake. To conclude, we try to match our model with the observations of a falling lake surface from Ref. [1]. Figure 6 shows how the lake depth drops for the parameters given in Table 1 and \( C = 2.0 \), meaning that the creep opening is twice what would be the initial elastic opening of the crack–crevasse system if subjected to hydrostatic water pressure

period of most rapid drainage but are unable to match the gradual onset of drainage shown in the data.

6 Conclusions

We have reviewed understanding of the coupled fluid and solid mechanics underlying an important class of natural hydraulic
fractures, involving rapid lake drainages through and under ice sheets and glaciers by turbulently flowing meltwater.

Our particular focus was on the 2006 rapid drainage event at a well-instrumented supraglacial lake, of $-44 \times 10^6$ m$^3$ volume, on the Greenland ice sheet. Once rapid drainage began, the lake drained into the ice within 1.0–1.5 hr.

We showed that our modeling of the drainage time has good correspondence with observational constraints on the rapidity of drainage. Although it is reasonable to assume that the ice responds elastically on such a short time scale, we noted that there was $\sim 16$ hr of slow drainage (shown by $\sim 15$ mm/hr fall in lake level), before the breakout to rapid drainage.

Therefore, using standard temperature dependent power law creep modeling of ice, we quantified possible slow creep opening over that 16 hr period, due to hydrostatic pressurization of a vertical cracklike crevasse system, 2.7 km long as exposed at the surface, which connects the lake bottom to the glacial bed, 1 km below. The crevasse is presumed to be the main conduit for the water.

A creep parameter $C$ was introduced, giving the ratio of the creep opening to what would be the elastic opening of that same vertical crack–crevasse if its surfaces were loaded by hydrostatic fluid pressure. Values of $C$ in the range of 1.5–2.0 were shown to give an excellent fit to the observations; the former best predicts the average flux of water out of the lake (Fig. 5, lower right), whereas the latter best fits the maximum observed rate of lake level descent (Fig. 6).

Using data on the temperature dependence of creep, we concluded that ice in the vicinity of the lake would have to respond as if it had a temperature in the range of $-7.0$ to $-5.0$ °C to produce such values of $C$.

GPS measurements were also reported in Fig. 2(c) of Ref. [1] and it is not yet clear that our present style of modeling, even if improved in sophistication, can fully explain them. They show that the ice sheet was moving primarily to the west with a slight northern trend (at 6% of the westward motion) prior to the rapid drainage event. The event itself caused a rapid 0.8 m displacement of the GPS site to the north, which was followed by its gradual return south over the next 2 days, after which the primarily westward pre-event motion was recovered.

Based on that pre-event motion, we must assume that the shear traction on the base of the ice sheet was primarily eastward-directed prior to the hydraulic fracture and lake drainage. Further, if the vertical crack/crevasse system (Fig. 1) became highly pressurized over a multihour period before the basal hydraulic fracturing, the deformation caused by that pressurization was resisted not just by the stress-dependent creep flow within the ice sheet (which we have modeled here), but also by the development of a component of traction at the base of the ice sheet, which would be a traction component in a direction approximately orthogonal to the pre-event eastward traction, and its development is expected to attenuate the short-term creep motion of the ice sheet in a manner consistent with the minimal observed northward displacements at the GPS site prior to the hydraulic fracture. At present, we have no good procedure to describe the process and its effect on the creep opening, which we have modeled here as if there was no basal shear resistance.

However, an assumption of no (or negligible) basal shear resistance is reasonable over the part of the base that is being hydraulically fractured, and hence for assessing the elastic part of the response to crack/crevasse opening. It is clear that a fuller analysis of the basal creep mechanics and shear resistance, in a manner which also rationalizes the GPS observations, is a significant goal for future clarification.

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Nomenclature

- $A_{\text{init}}$ = initial lake surface area
- $A(x, T)$ = area of lake cross section at any depth $z$
- $C$ = ratio of creep to elastic opening of hydrostatically pressurized vertical crevasse
- $D$ = initial lake depth at deepest point
- $E$ = Young’s modulus
- $E' = E/(1 - \nu^2)$ = effective modulus in plane strain
- $f$ = Darcy–Weischbach friction factor
- $g$ = gravitational acceleration
- $H$ = uniform ice sheet thickness ($-1$ km)
- $h_{\text{avg}}(t)$ = average opening gap of basal fracture
- $H_{\text{lake}}$ = lake water level
- $h(x, t)$ = opening gap of basal fracture
- $k$ = Nikuradse wall roughness amplitude
- $K_i$ = mode I fracture toughness of ice
- $K_{\text{ref}}(x; z)$ = Erdogan elasticity kernels
- $L(t)$ = half-length of basal fracture
- $n$ = power law creep exponent
- $P_{\text{inlet}}(l)$ = fluid pressure at basal entry $x = 0$
- $p(x, t)$ = basal fluid pressure distribution
- $Q$ = maximum volumetric discharge
- $Q_{\text{init}} = \text{volumetric discharge rate into basal fracture}$
- $Q_{\text{vert}} = \text{volumetric discharge rate through vertical crevasse}$
- $r$ = radius of the lake at a given depth
- $R$ = radius of the basal fracture when assumed circular
- $Re$ = Reynolds number
- $s_{\text{dev}} = \text{deviatoric stress components}$
- $t$ = time
- $T$ = temperature
- $U = \text{thickness averaged fluid velocity in basal fracture}$
- $U_{\text{up}} = \text{fracture propagation velocity} (= dL/dt)$
- $U_{\text{vert}} = \text{thickness averaged fluid velocity in crevasse}$
- $V_{\text{init}} = \text{volume of the lake}$
- $V(t) = \text{volume in the lake}$
- $W = \text{horizontal length of vertical crack–crevasse}$
- $x = \text{x-axis}$
- $y = \text{y-axis}$
- $z = \text{z-axis}$
- $Z_{\text{ref}} = \text{GPS-recorded ice uplift}$
- $\Delta = \text{constant determined using observations}$
- $\Delta p = \text{excess ice pressure} p_{\text{inlet}} - \sigma_f$
- $\Delta h(t) = \text{average opening gap of vertical crevasse}$
- $\Delta d(x, z; t) = \text{displacement discontinuities}$
- $\Delta d_c = \text{average creep crevasse opening}$
- $\Delta d_e = \text{average elastic crevasse opening}$
- $\kappa(n) = \text{numerical correction factor depending on n}$
- $\nu = \text{Poisson ratio}$
- $\rho = \text{mass density of water}$
- $\rho_{\text{ice}} = \text{mass density of ice}$
- $\sigma_f = \text{ice overburden pressure} p_{\text{inlet}} g H$
- $\sigma_{\text{dev}}(x, z; t) = \text{stress components}$
- $\tau = \text{Mises equivalent shear stress}$
- $\tau_{\text{wall}} = \text{wall shear stress}$

References


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